

ALUMINCO S.A. TEST REPORT

SCOPE OF WORK

STRUCTURAL PEFORMANCE TESTING ON THE *ELXIS RAIL - TYPE B* ALUMINUM AND GLASS GUARDRAIL SYSTEM

REPORT NUMBER

13376.02-119-19-R0

TEST DATE(S)

05/14/18 - 07/18/18

ISSUE DATE

08/28/18

RECORD RETENTION END DATE

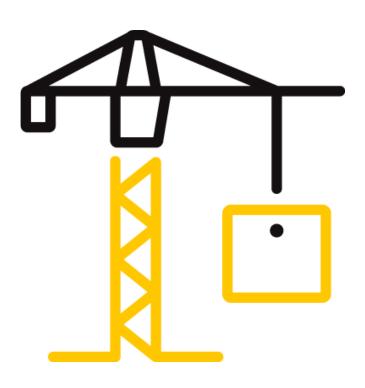
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PAGES

14

DOCUMENT CONTROL NUMBER

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TEST REPORT FOR ALUMINCO S.A.

Report No.: I3376.02-119-19-R0

Date: 08/28/18

REPORT ISSUED TO

ALUMINCO S.A. ENGINEERING DIVISION

Inofita Viotia, 32011 Greece

SECTION 1

SCOPE

Intertek Building & Construction (B&C) was contracted by Aluminco S.A., Viotia, Greece to perform structural performance testing in accordance with the 2018 IBC on their 50 in wide (nominal) by 47 in high (nominal) Elxis Rail - Type B aluminum and glass guardrail system. All tests performed were to evaluate structural performance of the guardrail assembly to carry and transfer imposed loads to the supporting structure. The test specimens evaluated included the infill, rails, rail brackets, and support posts. Anchorage of support posts to the supporting structure is not included in the scope of this testing and would need to be evaluated separately.

Results obtained are tested values and were secured by using the designated test method(s). Testing was conducted at the Intertek B&C test facility in York, PA. This report does not constitute certification of this product nor an opinion or endorsement by this laboratory.

SECTION 2

SUMMARY OF TEST RESULTS

The specimen met the 2018 IBC design load performance requirements.

For INTERTEK B&C:

Alva R. Baker COMPLETED BY: Technician III TITLE: **SIGNATURE:** 08/28/18 DATE:

REVIEWED BY: TITLE: **SIGNATURE:** DATE:

V. Thomas Mickley, Jr., P.E. Senior Staff Engineer

08/28/18

ARB:vtm/aaa

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Version: 02/09/18 Page 2 of 14 RT-R-AMER-Test-2846



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TEST REPORT FOR ALUMINCO S.A.

Report No.: I3376.02-119-19-R0

Date: 08/28/18

SECTION 3

TEST METHOD(S)

The specimens were evaluated in accordance with the following:

2018, International Building Code®, International Code Council

2018, International Residential Code®, International Code Council

Structural tests were performed according to Chapter 17 (Structural Tests and Special Inspections) of IBC 2018.

Limitations

All tests performed were to evaluate structural performance of the guardrail assembly to carry and transfer imposed loads to the supporting structure. The test specimens evaluated included the rails and their connection to the support posts, the glass panels and the support posts.

Anchorage of the support posts to the supporting structure is not included in the scope of this testing and would need to be evaluated separately.

SECTION 4

MATERIAL SOURCE/INSTALLATION

Test samples were provided by the client. Representative samples of the test specimen(s) will be retained by Intertek B&C for a minimum of four years from the test completion date.

The 50 in wide (nominal) by 47 in high (nominal) guardrail assembly was installed and tested as a single railing section by directly securing the posts onto the surface of rigid steel channels (to simulate anchorage into concrete), which allowed the posts to rotate under load. Transducers mounted to an independent reference frame were located to record movement of reference points on the guardrail system components (ends and mid-point) to determine net component deflections. See photographs in Section 11 for individual test setups.

SECTION 5

EQUIPMENT

The guardrail was tested in a self-contained structural frame designed to accommodate anchorage of the guardrail assembly and application of the required test loads. The specimens were loaded using an electric winch mounted to a rigid steel test frame. High strength steel cables, nylon straps, and load distribution beams were used to impose test loads on the specimens. Applied load was measured using an electronic load cell located in-line with the loading system. Electronic linear motion transducers were used to measure deflections.

Version: 02/09/18 Page 3 of 14 RT-R-AMER-Test-2846



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TEST REPORT FOR ALUMINCO S.A.

Report No.: I3376.02-119-19-R0

Date: 08/28/18

SECTION 6

LIST OF OFFICIAL OBSERVERS

NAME	COMPANY
Alva R. Baker	Intertek B&C
Emily C. Riley	Intertek B&C

SECTION 7

TEST PROCEDURE

Each test specimen was inspected prior to testing to verify size and general condition of the materials, assembly, and installation. No potentially compromising defects were observed prior to testing.

An initial load, not exceeding 50% of design load, was applied and transducers were zeroed. Load was then applied at a steady uniform rate until reaching 2.0 times design load in no less than 10 seconds. After reaching 2.0 times design load, the load was released. After allowing a minimum period of one minute for stabilization, load was reapplied to the initial load level used at the start of the loading procedure, and deflections were recorded and used to analyze recovery. Load was then increased at a steady uniform rate until reaching 2.5 times design load (loads on aluminium components / 4.0 times design load (loads on glass components) or until failure occurred. The testing time was continually recorded from the application of initial test load until the ultimate test load was reached.

Deflection and permanent set were component deflections relative to their end-points; they were not overall system displacements. All loads and displacement measurements were horizontal, unless noted otherwise.

Version: 02/09/18 Page 4 of 14 RT-R-AMER-Test-2846



Telephone: 717-764-7700 Facsimile: 717-764-4129 www.intertek.com/building

TEST REPORT FOR ALUMINCO S.A.

Report No.: I3376.02-119-19-R0

Date: 08/28/18

SECTION 8

TEST SPECIMEN DESCRIPTION

Aluminco S.A. provided the fully-assembled test specimens with the following details:

PRODUCT	Elxis Rail - Type B
ТҮРЕ	Aluminum and glass guardrail system
MATERIAL	Unspecified aluminum alloy
OVERALL DIMENSIONS	50 in / 50-3/8 in wide (inside of post to inside of post)
	51 in (center of post to center of post)
	47-3/8 in high (bottom of base plate to top of top rail)
TOP RAIL	11/16 in high by 2-15/16 in wide aluminum extrusion with 0.09 in
	wall and a 1-3/16 in high by 3-9/16 in wide by 0.06 in wall snap-fit
	contoured extruded aluminum cover
BOTTOM/INTERMEDIATE	1-1/8 in high by 1-5/16 in wide aluminum extrusion with 0.09 in
RAIL	wall
GLASS PANELS	7/16 in thick laminated glass constructed from two sheets of 3/16
	in thick clear tempered glass and an 0.060 in thick interlayer
BOTTOM/INTERMEDIATE	1-9/16 in high by 3-15/16 in wide by 13/16 in thick extruded
RAIL BRACKET	aluminum bracket
SUPPORT POST	2-9/16 in deep by 13/16 in / 1-1/8 in wide T-shaped aluminum
	extrusion with 0.14 in wall and three raceway channels running the
	entire length of the post.
BASE PLATE	4 in deep by 4-3/8 in wide by 7/16 in / 9/16 in thick extruded
	aluminum base plate with two 1/2 in diameter countersunk holes
	for attachment to substructure, three 1/4 in diameter countersunk
	holes for attachment of support post to base plate and two 1/4 in
	diameter tapped holes for leveling screws

Version: 02/09/18 Page 5 of 14 RT-R-AMER-Test-2846



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TEST REPORT FOR ALUMINCO S.A.

Report No.: I3376.02-119-19-R0

Date: 08/28/18

Fastening Schedule

TOP RAIL TO POST	Three 1/4-14 in by 2 in long Phillips drive, flat head stainless steel
ATTACHMENT	screws through top rail into post screw chases
GLASS PANEL TO	Slip-fit into rubber gaskets
BOTTOM/INTERMEDIATE	
RAILS	
BOTTOM/INTERMEDIATE	One 1/4-14 in by 2 in long Phillips drive, flat head stainless steel
RAIL BRACKET TO POST	screw
BOTTOM/INTERMEDIATE	Two #12-24 by 1/2 in long set screws
RAIL BRACKET TO	
BOTTOM/INTERMEDIATE	
RAIL	
BASE PLATE TO POST	Three 1/4-14 in by 3-1/4 in long Phillips drive, flat head stainless
	steel screws through base plate into post screw chases
BASE PLATE TO STEEL	Two M10 x 50 mm long allen drive flat head socket cap bolts and
CHANNEL	nylon lock nuts

SECTION 9

TEST RESULTS

Key to Test Results Tables:

Load Level: Target test load

<u>Test Load</u>: Actual applied load at the designated load level (target).

<u>Elapsed Time (E.T.)</u>: The amount of time into the test with zero established at the beginning of the loading procedure.

Specimen No. 1

Test No. 1 - 05/14/18

DESIGN LOAD: 200 lb Concentrated Load at Midspan of Top Rail

LOAD LEVEL	TEST LOAD	E.T.	DISPLACEMENT (in)			
LOAD LEVEL	(lb)	(min:sec)	END	MID	END	NET ¹
Initial Load	39	00:00	0.00	0.00	0.00	0.00
2.0x Design Load	400	00:31	4.09	4.14	3.80	0.20
Initial Load	40	02:51	2.57	2.41	2.19	0.03
85% Recovery from 2.0 x Design Load						
2.5x Design Load	502	03:41	Achieved	Load with	out Failure	

¹ Net displacement was mid-rail displacement relative to the rail at the support posts.



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TEST REPORT FOR ALUMINCO S.A.

Report No.: I3376.02-119-19-R0

Date: 08/28/18

Specimen No. 2

<u>Note</u>: A new assembly was used because specimen No. 1 was over stressed from previous loading conditions and could not sustain the test load.

Test No. 1 - 05/16/18

DESIGN LOAD: 200 lb Concentrated Load at End of Top Rail (Rail to Post Connection/Post)

LOAD LEVEL	TEST LOAD (Ib)	E.T. (min:sec)	RAIL END/POST DISPLACEMENT (in)
Initial Load	38	00:00	0.00
2.0x Design Load	400	00:35	6.12
Initial Load	40	02:41	2.98
51% Recovery from 2	2.0 x Design Load		
2.5x Design Load	502	03:20	Achieved Load without Failure

Test No. 2 - 05/16/18

DESIGN LOAD: 50 lb / 1 Square ft at Center of In-fill

LOAD LEVEL	TEST LOAD	E.T.	DISPLACEMENT (in)			
LOAD LEVEL	(lb)	(min:sec)	END	MID	END	NET ¹
Initial Load	9	00:00	0.00	0.00	0.00	0.00
2.0x Design Load	100	00:09	0.34	0.25	0.07	0.05
Initial Load	10	01:59	0.01	0.01	0.00	0.01
80% Recovery from 2.0 x Design Load						
4.0x Design Load	202	02:33	Achieved	Load with	out Failure	

¹ Net displacement was the infill displacement relative to its top and bottom.

Test No. 3 - 05/16/18

DESIGN LOAD: 50 lb / 1 Square ft at Bottom of In-fill

LOAD LEVEL	TEST LOAD	E.T.	RAIL DISPLACEMENT (in)			
LOAD LEVEL	(lb)	(min:sec)	END	MID	END	NET 1
Initial Load	10	00:00	0.00	0.00	0.00	0.00
2.0x Design Load	102	00:07	0.09	0.10	0.09	0.01
Initial Load	10	01:37	0.00	0.00	0.00	0.00
100% Recovery from 2.0 x Design Load						
4.0x Design Load	200	02:02	Achieved	Load with	out Failure	

¹ Net displacement was the bottom rail displacement relative to its ends.



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TEST REPORT FOR ALUMINCO S.A.

Report No.: I3376.02-119-19-R0

Date: 08/28/18

Test No. 4 - 05/16/18

DESIGN LOAD: 50 lb / 1 Square ft at Bottom Corners of Infill, Adjacent to Bracket Connections

I IOAD I EVEL 4	TEST LOAD	E.T.	RAIL DISPLACEMENT (in)			
	(lb)	(min:sec)	END	MID	END	NET 1
Initial Load	24	00:00	0.00	0.00	0.00	0.00
(2.0x Design Load) x 2	204	00:35	0.02	0.08	0.02	0.06
Initial Load	36	02:09	0.00	0.01	0.00	0.01
83% Recovery from 2.0 x Design Load						
(4.0x Design Load) x 2	418	02:40	Achieved	Load with	out Failure	

¹ Net displacement was the bottom rail displacement relative to its ends.

Specimen No. 3 - Loading from Opposite Direction

<u>Note</u>: A new assembly was used because specimen No. 2 was over stressed from previous loading conditions and could not sustain the test load.

Test No. 1 - 07/18/18

DESIGN LOAD: 200 lb Concentrated Load at End of Top Rail (Rail to Post Connection/Post)

LOAD LEVEL	TEST LOAD (lb)	E.T. (min:sec)	RAIL END/POST DISPLACEMENT (in)
Initial Load	39	00:00	0.00
2.0x Design Load	400	00:40	8.22
Initial Load	40	02:50	4.84
41% Recovery from	n 2.0 x Design Lo	ad	
2.5x Design Load	502	03:46	Achieved Load without Failure

SECTION 10

CONCLUSION

Using performance criteria of withstanding an ultimate load of 2.5 (4.0 for glass infill) times design load, the test results substantiate compliance with the design load requirements of the referenced building codes for the 50 in wide (nominal) by 47 in high (nominal) railing assembly (*Elxis Rail - Type B*) reported herein. Anchorage of support posts to the supporting structure is not included in the scope of this testing and would need to be evaluated separately.

Version: 02/09/18 Page 8 of 14 RT-R-AMER-Test-2846

² A spreader beam was used to impose loads on both ends of the infill system; therefore, loads were doubled.



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TEST REPORT FOR ALUMINCO S.A.

Report No.: I3376.02-119-19-R0

Date: 08/28/18

SECTION 11

PHOTOGRAPHS



Photo No. 1
Concentrated Load Test at Mid-Span of Top Rail



Photo No. 2
Concentrated Load at End of Top Rail (Rail to Post Connection/Post)

Version: 02/09/18 Page 9 of 14 RT-R-AMER-Test-2846



Telephone: 717-764-7700 Facsimile: 717-764-4129 www.intertek.com/building

TEST REPORT FOR ALUMINCO S.A.

Report No.: I3376.02-119-19-R0

Date: 08/28/18



Photo No. 3
In-Fill Load Test at Center of Glass In-fill



Photo No. 4
In-Fill Load Test at Bottom of Glass In-fill



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TEST REPORT FOR ALUMINCO S.A.

Report No.: I3376.02-119-19-R0

Date: 08/28/18

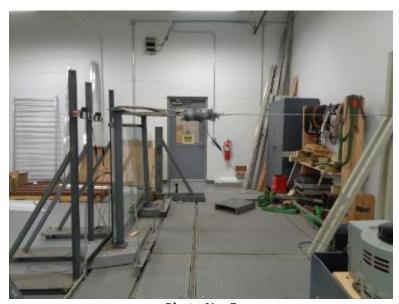


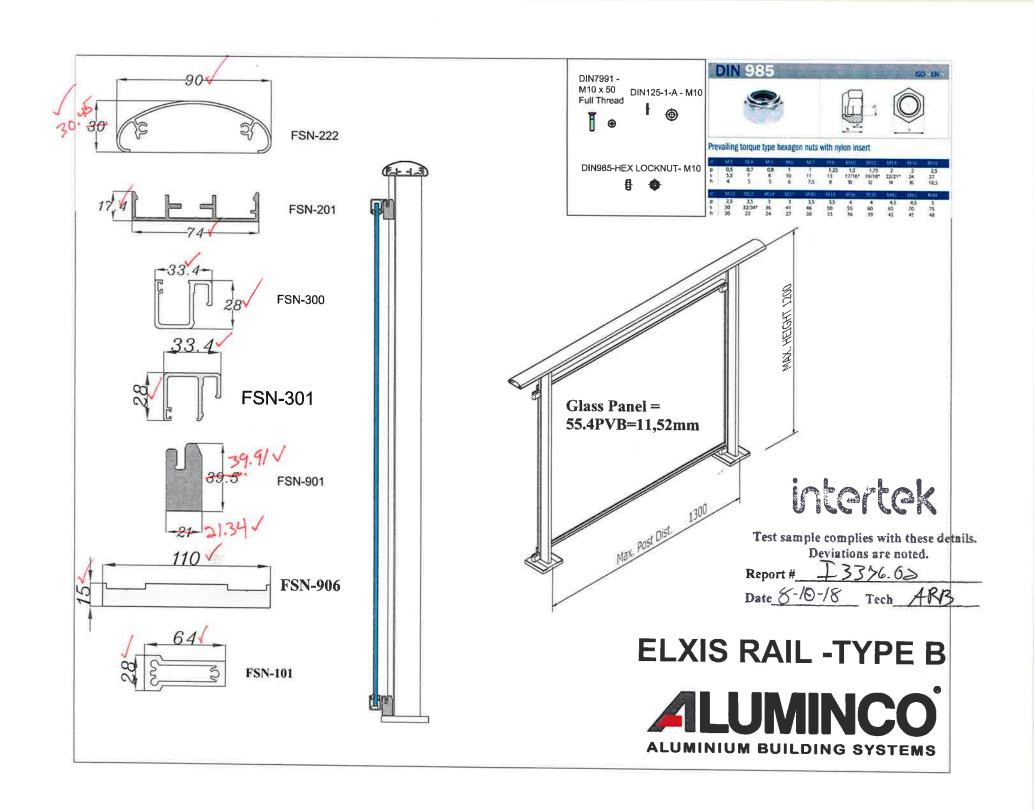
Photo No. 5
Concentrated Load at End of Top Rail (Rail to Post Connection/Post)
Loading from Opposite Direction

SECTION 12

DRAWINGS

The "As-Built" drawings for the *Elxis Rail - Type B* which follow have been reviewed by Intertek B&C and are representative of the project reported herein. Project construction was verified by Intertek B&C per the drawings included in this report. Any deviations are documented herein or on the drawings.

Version: 02/09/18 Page 11 of 14 RT-R-AMER-Test-2846



ELXIS RAIL TYPE B LUMINCO ALUMINIUM BUILDING SYSTEMS

-1400--1300-1400-1200 1400 x 1029 (Y) -> MAX. 55.4PVB=11.52mm RODEX 1294 1300-

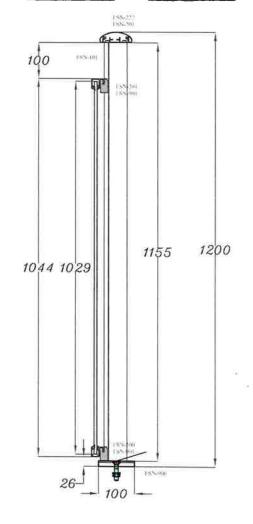
intertek

Test sample complies with these details.

Deviations are noted.

Report # 133 76.62

Date 8-10-18 Tech ARB





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TEST REPORT FOR ALUMINCO S.A.

Report No.: I3376.02-119-19-R0

Date: 08/28/18

SECTION 13

REVISION LOG

REVISION #	DATE	PAGES	REVISION
0	08/28/18	N/A	Original Report Issue
		-	<u> </u>